ESTIMATION OF LIFE OF RIPPERTIP DUE TO WEAR

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1. INTRODUCTION

One of the most important problems in mechanical excavation of rock mass by means of ripping operations is to estimate precisely the life of tip due to wear and to establish the reasonable plan for excavation work based on fair judgements. The ability of excavation of rippertip against rock mass has already been evaluated by means of its longitudinal elastic wave velocity¹⁾. But, the hardness or ease of ripping operation for rock mass must be considered to be determined not only by maximum power of excavation of bulldozer but also by the life of tip due to wear. So far, we have studied about the characteristics of shape variation of tip due to wear and abrasiveness of rock^{2),3),4)}, and about correlations between several properties of rock⁵⁾.

Here, the wear life of rippertip excavating several rock masses has been analysed. For this purpose, 11 series of wear tests of tip were executed at the different ripping operation sites respectively. These ripping operation sites have been selected as comparatively severe wear zone for rippertip excavating rock mass. In these cases, all ripping operations were executed by the same kinds of bulldozer of which maximum traction forces were nearly equal to 30 ton and vehicle forms, types and weight were almost the same. In these field tests, the physical properties of rock mass were investigated, and the variation of amount of wear or wear length of tip were measured respectively for several ripping operation time. As the results, it has been cleared that the index of rock mass strength for wear which is determined by uniaxial compressive strength, coefficient of crack, hard rock forming minerals content, apparent specific gravity, amount of Los Angeles abrasion, absorption and Shore hardness, determines the wear life of rippertip. And

the aim of this study is to determine the equations which estimate the wear life of rippertip.

2. CHARACTERISTICS OF ROCK MASS

In these 11 sites of ripping operations named A, B, C, ... and K, several in-situ tests for each rock mass were executed, and, at the same site, rock specimens were sampled for several laboratory rock tests. Table 1 shows these test results. As laboratory rock tests, apparent specific gravity G , specific gravity G_s , natural water content W , absorption W' , uniaxial compressive strength σ_{oo} , Shore hardness H_s , amount of Los Angeles abrasion U , longitudinal elastic wave velocity V_1 and porosity n_1 of non-fissured rock sample were measured respectively. Each laboratory test was executed for non-fissured rock sample in which any crack or fissure is not observed with the naked eye. The amount of Los Angeles abrasion was measured by means of the method prescribed in JIS A 1121 for 12 steel balls, about 8000 g rock block specimens, and the number of rotations 1000. The porosity of rock specimen n_1 was calculated as $n_1 = (G_s - G)/G_s$, that is, the porosity of the inner parts of rock which is entirely isolated from the outside world is also calculated. As in-situ test of rock mass, longitudinal elastic wave velocity of rock mass V_2 was measured. Then the coefficient of crack of rock mass C_r ⁶⁾ was calculated by use of Eq. (1), from V_2 for fissured rock mass and V_1 for non-fissured rock specimen.

$$C_r = 1 - (V_2/V_1)^2 \dots\dots\dots (1)$$

And, the other method of quantification of discontinuities of rock and rock mass is proposed by Fourmaintraux. D ⁷⁾ as "Rock Quality Index IQ " which is defined from the longitudinal elastic wave velocity measurements V_{LM} in rock sample or rock mass divided by the elastic wave velocity V_{LC} calculated from several experimental data. It makes possible the detection, quantification and description of discontinuities of rock or rock mass as shown in Eq. (2).

$$IQ (\%) = (V_{LM}/V_{LC}) \times 100 \dots\dots\dots (2)$$

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Table 1 Physical properties of rock mass and rock specimen.

Site of Sampling	A	B	C	D	E	F	G	H	I	J	K
Name of Rock	Clay-Slate	Horn-blende Gabbro	Shale	Serpentine	Sand Stone	Me'ta-morphic Andesite	Chert	Sandy Clay Slate	Granite	Chert	Clay Slate
Apparent Specific Gravity G	1.85	2.86	2.63	2.61	2.53	2.58	2.55	2.32	2.59	2.61	2.00
Specific Gravity G_s	2.62	2.89	2.73	2.67	2.61	2.63	2.65	2.47	2.63	2.64	2.23
Natural Water Content W (%)	2.14	0.17	0.09	0.66	0.37	1.94	1.36	3.70	0.27	0.70	5.79
Absorption W' (%)	1.50	0.89	1.44	1.96	2.75	3.08	2.84	9.13	0.81	0.89	18.25
Uniaxial Compressive Strength σ_{co} (kg/cm ²)	192	1192	250	550	370	344	1114	81	840	736	120
Shore Hardness H_s	21.9	76.6	96.2	71.2	82.4	48.9	91.4	8.5	83.3	87.8	30.2
Amount of Los Angeles Abrasion U (%)	25.1	12.2	26.5	33.0	35.6	24.0	39.8	39.3	35.3	20.6	20.5
Elastic Wave Velocity of Non-fissured Rock Sample V_1 (m/sec)	750	2488	1500	2578	3840	3645	2868	1971	3679	3247	2475
Elastic Wave Velocity of Rock Mass V_2 (m/sec)	640	1093	950	2060	1143	1315	1494	1348	1743	1650	1125
Coefficient of Crack C_r	0.272	0.807	0.599	0.362	0.911	0.870	0.729	0.532	0.776	0.742	0.793
Rock Quality Index of Non-fissured Rock Sample IQ_1 (%)	52.96	98.35	94.14	96.42	95.10	96.96	93.84	90.29	97.57	98.42	83.47
Rock Quality Index of Rock Mass IQ_2 (%)	45.20	43.20	59.64	77.04	28.31	34.98	48.89	61.75	46.22	50.02	37.94
Porosity of Non-fissured Rock Sample n_1 (%)	29.40	1.03	3.66	2.24	3.06	1.90	3.85	6.07	1.52	0.99	10.33
Porosity of Rock Mass n_2 (%)	29.74	3.48	5.19	3.10	6.03	4.65	5.85	7.34	3.80	3.14	12.35
$K \cdot K_0^2$ Value	0.464	0.414	0.262	0.177	0.473	0.474	0.991	0.131	0.691	1.591	0.179
Index of Rock Mass Strength for Wear σ_c (kg/cm ²)	65	95	26	62	16	21	299	5	130	302	4
Mineral Composition (%)											
Quartz (%)	38.0	0	4.1	0.2	31.0	0	18.0	0.1	41.9	48.0	0
Orthoclase (%)	0	0	0	0	29.0	0	0	0	0.5	0	38.0
Plagioclase (%)	62.0	58.8	0	0	1.0	47.0	0	99.7	54.6	2.0	62.0
Mica (%)	0	3.3	23.9	0	2.0	0	0	0	2.2	0	0
Groundmass (%)	0	0	37.2	0	23.0	47.0	82.0	0	0	50.0	0
Olivine (%)	0	0	0	82.1	0	0	0	0	0	0	0
Magnetite (%)	0	0	4.7	17.7	0	1.0	0	0.2	0	0	0
Calcite (%)	0	0	0	0	0	0	0	0	0.8	0	0
Chlorite (%)	0	0	30.1	0	0	0	0	0	0	0	0
Amphibole (%)	0	37.9	0	0	0	0	0	0	0	0	0
Detritus (%)	0	0	0	0	14.0	5.0	0	0	0	0	0

In general, the porosity n of rock or rock mass consists of porosity of fissure n_f and porosity of pore n_p . From several experimental relationships between total porosity n and elastic wave velocity V_L in porous rock or cracked rock, the elastic wave velocity V_{LC} calculated for zero porosity of rock is determined. From Fourmaintraux Diagram, rock quality index IQ_1 for non-fissured rock sample is calculated by use of n_1 ($=n_p$, $n_f=0$), and rock quality index IQ_2 for fissured rock mass is also calculated by use of V_{LC} and V_2 . Then, the total porosity n_2 of fissured rock mass is determined by n_1 , IQ_1 and IQ_2 as shown in Table 1. Furthermore, the relations of rock quality index IQ_2 and total porosity n_2 of fissured rock mass A, B, C, ... and K are plotted in Fourmaintraux Diagram as

shown in Fig. 1, and these points show the rock quality index, porosity of fissure and porosity of pore respectively.

Fig. 1 Relations between rock quality index IQ_2 and porosity n_2 for fissured rock mass.

Next, the mineral compositions were analysed for each rock specimen. The contents of quartz, orthoclase, plagioclase, mica etc. were measured by use of polarization microphotograph.

Now, the principal properties of rock mass which influence the amount of wear of rippertip are not evaluated by the compressive strength of rock specimen only, but they must be synthetically evaluated from several properties of rock mass. The degree of crack in rock mass, content of hard rock forming mineral, degree of weathering expressed by apparent specific gravity, amount of Los Angeles abrasion, absorption, and Shore hardness influence also the amount of wear of tip, even if the quality, size and shape of tip are the same.

Here, as an index which appreciates the strength of rock mass against wear of rippertip, "Index of Rock Mass Strength for Wear" σ_o is defined as follows;

$$\left. \begin{aligned} \sigma_o &= K \cdot K_0^2 \sigma_{co}(1 - C_r) \\ K &= \frac{1}{4}(K_1 + K_2 + K_3 + K_4) \end{aligned} \right\} \dots\dots\dots (3)$$

where K_0 = (Content of Quartz and Groundmass)
 + (Content of hard Feldspar etc.) $\times 0.5$
 σ_{co} = Uniaxial compressive strength of non-fissured rock sample
 C_r = Coefficient of crack of rock mass
 K_1 = (Apparent specific Gravity)/2.60
 K_2 = 30.0/(Amount of Los Angeles Abrasion)
 K_3 = 2.00/(Absorption)
 K_4 = (Shore hardness)/60.0

This index of rock mass strength for wear has been given by product of independent factors which have been selected out of several properties of rock mass having important relations with the amount of wear of tip. That is, the amount of wear of tip is assumed to be proportional to the compressive strength of rock mass which is also assumed to be defined as $\sigma_{co}(1 - C_r)$ for fissured rock mass, because the amount of wear of metal is proportional to the contact pressure between metal and rock materials. And, the index of rock mass strength for wear is defined as product of the compressive strength and coefficient of modification $K \cdot K_0^2$. Here, $1 - C_r = (V_2/V_1)^2$ means also that "Coefficient of damage of rock mass" V_2/V_1 ⁸⁾ is more higher evaluated than the other factors. And, K_0^2 means that the content of quartz, groundmass, and hard feldspar grains or other rock forming minerals of which hardness are larger than the hardness of rippertip is more higher evaluated, and 50% of the content of hard feldspar etc. is assumed to be contributed in the wear of tip, and K is defined as an arithmetical average value of K_1, K_2, K_3

and K_4 as shown in Eq. (3). K_1, K_2, K_3 and K_4 is expressed by non-dimensional number. And each number 2.60, 30.0, 2.00, and 60.0, is given for wide range of rock properties by the average value of apparent specific gravity, amount of Los Angeles abrasion, absorption and Shore hardness, respectively. Furthermore, the dimension of σ_o is the same as that of σ_{co} i.e. kg/cm². The value of $K \cdot K_0^2$ and index of rock mass for wear σ_o calculated from Eq. (3) for each site of ripping operation are shown in Table 1 respectively. And this equation of index of rock mass strength has been made by several trial and errors, but its expression is proved to be actually appropriate by the relation between amount of wear of rippertip and index of rock mass strength for wear as shown in later section.

3. VARIATION OF WEAR AMOUNT AND WEAR LENGTH OF TIP

All of the rippertips tested in the field of each site A, B, C, ..., and K have the same size, shape and quality. Fig. 2 shows the size of tip, which is selected as generally used tip. The total length is 375 mm, the width of pointed end is 88 mm, and the initial weight is about 15 kg. The average tensile strength of the material of tip is 156.5 kg/mm², the elongation is 6.2%, the contraction of area is 27.8%, the Charpy impact value is 5.9 kgm/cm², the Vicker's hardness is 402 ± 61.5 and the Shore hardness is about 55.1. And the tip is constituted by specialized alloy steel with the chemical composition C: 0.27%, Si: 0.29%, Mn: 0.77%, P: 0.018%, S: 0.018%, Ni: 0.61%, Cr: 0.51%, and with a feathered structure of uniformly distributed fine carbide in a bainite modification.

Fig. 2 Initial dimension and shape of rippertip.

The summarization of wear test results which the variations of wear length of tip with time were measured for each site of ripping operation is shown in Fig. 3. The wear length of pointed end of tip was measured by use of slide calipers as the

Fig. 3 Relations between wear length of pointed end of rippertip L and ripping operation time T (Numbers show σ_c values).

difference between initial length and the measured length for each time. And the length of tip was measured along the central axis, from its fixed point to the end point. And, the ripping operation time showed in abscissa is the time required for practically reciprocating motion of bulldozer for ripping operation only, except for dozing or pushing operation, and was measured directly at each site by use of stop watch.

As shown in this figure, the variation of wear length is characterized respectively by the properties of rock mass. The higher the index of rock mass strength for wear, the greater the wear length of tip increases due to impact load acted on tip at the time of excavation. The variation of wear length of tip with ripping operation time disperses because the conditions of ripping operation are not constant for each site. But, it seems to be able to approximate some linear relationship between the wear length of tip L and the ripping operation time T . Assuming that the linear equation $L=aT$ is formed, the most probable value of coefficient " a " and coefficient of correlation " r_a " are determined by means of principle of least squares, and are shown in Table 2.

Next, the wear amount of rippertip was measured precisely in the given field. Fig. 4 shows the summarization of wear test results. The variation of wear amount of rippertip with time were measured at the same time of measuring the wear length of pointed end of tip. These amount of wear are calculated as weight loss that is the difference between initial weight and the weight of tip which was measured for given operation time by means of removing it from bulldozer one by one. For measuring the weight loss of tip, a platform scale

Table 2 Most probable value of coefficient a , b and coefficient of correlation r_a , r_b , for wear length $L=aT$ and amount of wear $M=bT$ respectively (T is ripping operation time).

Site	L (mm) = $a \cdot T$ (hr)		M (g) = $b \cdot T$ (hr)	
	a	r_a	b	r_b
A	2.54	0.996	140.0	0.995
B	6.32	0.945	278.7	0.983
C	1.15	0.997	51.3	0.996
D	2.52	0.973	119.5	0.994
E	0.70	0.986	40.7	0.972
F	0.73	0.990	46.7	0.906
G	67.90	0.997	2965.1	0.996
H	0.20	0.974	29.0	0.981
I	17.93	0.996	657.5	0.996
J	69.24	0.986	3023.2	0.995
K	0.23	0.953	27.1	0.959

Fig. 4 Relations between amount of wear of rippertip M and ripping operation time T (Numbers show σ_c values).

of maximum weighing 20 kg, sensitivity 1 g was used. In spite of the same rippertip, it is cleared that the characteristics of variation of wear amount of tip is dependent upon the properties of rock mass. The relations between the amount of wear of tip M and the ripping operation time T seem to be able to approximate some linear equations $M=bT$ for each site respectively. The most probable value of coefficient " b " and coefficient of correlation " r_b " are shown in Table 2.

4. CHARACTERISTICS OF SHAPE VARIATION OF TIP

As mentioned above, the higher the index of

rock mass strength for wear, the greater the wear length of tip increases due to impact load at the time of excavation. On the other hand, since almost all rock masses of small index of rock mass strength for wear are composed of rock materials of comparatively small abrasive strength⁹⁾, it is cleared that the wear length of pointed end of tip decreases relatively while the amount of wear of side wall of tip increases. That is, the compressive strength of rock materials of comparatively small abrasive strength are relatively small, then any impact load does not occur on the pointed end of tip. And, the high contact pressure acted on the pointed end of tip beyond the abrasive strength break down rock material instead of metal material, while the low contact pressure acted on the side wall of tip under the abrasive strength is contributed to wear metal material⁹⁾.

As representative examples, two kinds of shape variation of tip tested at site A and G are explained.

Photo. 1 General view of site during ripping operation (Site A).

Fig. 5 Shape variation of tip due to wear for some ripping operation time (Site A).

ed. Test site A is selected as impact wear of tip is predominant.

The ripping operation site A is formed of clay slate consisted of quartz and plagioclase only which has comparatively small abrasive strength. Photo. 1 shows the state of rock mass in this site A after some ripping operations by use of 2 tips, and several grooves of the trace of tip are clearly observed on the surface of rock mass. The occurrence of these grooves is generally observed in this kind of rock mass and is also observed in other operation site H. Fig. 5 (a) (b) show the plan and

(a)

(b)

(c)

Photo. 2 Examples of worn shape of rippertip (Site A).

elevation of shape variation of tip in this site A, which are traced respectively for each operation time. The numerals in this figure mean the ripping operation time expressed by hour. Photo. 2 (a) (b) (c) show the interesting shapes of tip which are worn out in this ripping operation site A. From these examples, it is considered that the ripperability for these rock masses of comparatively small abrasive strength of rock does not practically decrease with the progress of wear of tip, because the wear of side parts of tip is more remarkable than that of end pointed parts of tip and the shape of end pointed parts becomes sharp and slender.

Photo. 3 General view of site during ripping operation (Site G).

Photo. 4 Chert (Site G, Open Nikol $\times 59$).

Next, the state of ripping operation at site G by use of single tip is shown in Photo. 3. In this site, the life of rippertip is remarkably short since the tip is worn by abrasive action accompanying with impact load at the time of excavation for comparatively high fissured chert. Photo. 4 shows the polarization microphotograph of the chert in this site G. The main rock forming minearals

consist of groundmass and some quartz grains, and the rock mass is formed of sedimentary rock of single rock forming mineral of quartz. Fig. 6 (a) (b) show the plan and elevation of shape variation of tip in site G, which are traced respectively for each operation time. The numerals in this figure mean the ripping operation time expressed by minute. Photo. 5 (a) (b) show the shape of tip which is worn out in this ripping operation site. From these figures and photographs, it is cleared that the ripperability for these hard rock mass decreases gradually with the progress of wear of tip because the wear of end pointed parts of tip is more remarkable than that of side parts of tip and, as a whole, the shape of end pointed parts of tip becomes round.

Fig. 6 Shape variation of tip due to wear for some ripping operation time (Site G).

Photo. 5 Example of worn shape of rippertip (Site G). (a) plan (b) elevation

5. ESTIMATION OF LIFE OF TIP DUE TO WEAR

As previously mentioned, the index of rock mass strength for wear is largely influenced by the porosity of rock mass. For the purpose of analyzing this relation, the Fourmaintraux Diagram is modified in relation to the ratio of IQ_2 to IQ_1 as shown in right hand side of Fig. 7. This ratio IQ_2/IQ_1 is considered as V_2/V_1 . And this "Coefficient of damage of rock mass" V_2/V_1 means the square root of $1-C_r$. The relations of the ratio IQ_2/IQ_1 and total porosity n_2 of fissured rock mass A, B, C, ..., and K are plotted in this figure, from which the coefficient of damage of rock mass, porosity of fissure, and porosity of pore of given rock mass are easy to understand at a glance.

Fig. 7 Relations between porosity of rock mass n and ratio of rock quality index IQ_2/IQ_1 modified from Fourmaintraux Diagram, and contour lines of rock mass strength for wear σ_c .

Then, the contour lines of index of rock mass strength for wear are shown in the left hand side of Fig. 7 which is related between the ratio IQ_2/IQ_1 concerning the porosity of rock mass and the other factors $K \cdot K_0^2 \sigma_{c0}$. And, the points A, B, C, ..., and K are plotted from their in-situ and laboratory test results respectively. From these considerations, it is cleared that the relations between porosity or coefficient of crack of rock mass and $K \cdot K_0^2 \sigma_{c0}$ value have a significant meaning for the same index of rock mass strength for wear to understand the properties of rock mass and the life of tip due to wear.

From previous Fig. 3, the relation between wear length of pointed end of tip L and ripping operation time T is generally expressed by linear equation

$L=aT$. For almost all the cases, the tip should be exchanged into another new article when the wear length of pointed end of tip L reaches 160 mm. Then, assuming that the critical wear length L_c equals to 160 mm, the critical life of tip T_c can be calculated by $160/a$. Fig. 8 shows the relations between the critical life of tip T_c calculated from the measured wear length of pointed end of tip and index of rock mass strength for wear σ_c , which are plotted on logarithmic scale. The experimental equation obtained is as follows;

$$T_c = 6.40 \times 10^3 \sigma_c^{-1.26} \quad (r=0.904) \quad (4)$$

It is cleared that the coefficient of correlation is relatively low value, and their data are fairly scattered.

On the other hand, from previous Fig. 4, the relation between amount of wear of tip M and rip-

Fig. 8 Relations between critical wear life of rippertip T_c and rock mass strength for wear σ_c , calculated from measurements of wear length of tip.

ping operation time T is generally expressed by linear equation $M=bT$. For many cases, the tip should be exchanged into another new article when the amount of wear M reaches 7000 g. Then, assuming that the critical amount of wear M_c equals to 7000 g, the critical life of tip T_c can be calculated by $7000/b$. Fig. 9 shows the relations between the critical life of tip T_c calculated from the measured amount of wear of tip and index of rock mass strength for wear σ_c , which are plotted on logarithmic scale. These relations are shown by two straight line folded together at the index of rock mass strength for wear of about 43 kg/cm², which divides the rock mass into light wear zone and heavy wear zone. The experimental equations obtained are as follows;

Fig. 9 Relations between critical wear life of rippertip T_c and rock mass strength for wear σ_c , calculated from measurements of amount of wear of tip.

For light wear zone ($0 < \sigma_c \leq 43 \text{ kg/cm}^2$)

$$T_c = 4.10 \times 10^2 \sigma_c^{-0.998} \quad (r=0.996) \quad \dots\dots\dots (5)$$

For heavy wear zone ($\sigma_c > 43 \text{ kg/cm}^2$)

$$T_c = 2.40 \times 10^5 \sigma_c^{-2.03} \quad (r=0.998) \quad \dots\dots\dots (6)$$

It is cleared that the coefficients of correlation are remarkably high value, and these above equations are most useful for estimating the life of rippertip due to wear. And, this folded point seems to show that the plastic yielding of metal material of tip due to abrasive wear begins at this point of index of rock mass strength.

And, Eqs. (5) and (6) may be more higher appreciated rather than Eq. (4) as equations which estimate the wear life of rippertip, because of high coefficients of correlations. For estimating the wear life of tip, the in-situ test method of measuring amount of wear of tip is more valuable than that of measuring wear length of tip from the point of view of total energies of ripping operation work. Furthermore, the wear life of tip estimated from the in-situ test results shown in Fig. 4 agrees well with actual wear life of another tip measured at each site for even light wear zone. That is, the assumption of linear equation of $M=bT$ become clear to be right for the test method of measuring amount of wear of tip, while the in-situ test results do not reach the critical amount of wear for light wear zone.

6. CONCLUSION

For estimating the wear life of rippertip in mechanical excavation of comparatively hard rock

mass, many new informations are obtained from in-situ wear tests as follows;

(1) The characteristics of rock mass for wear of rippertip can be expressed by "Index of Rock Mass Strength for Wear" which is calculated by uniaxial compressive strength for non-fissured rock sample, coefficient of crack of rock mass, contents of quartz and hard rock forming minerals, and other factors related to wear of tip.

(2) The amount of wear and the wear length of tip increase approximately with linear relationship to the ripping operation time. Each slope varies respectively according to the index of rock mass strength for wear.

(3) For the rock mass which both the index of rock mass strength for wear and the coefficient of crack of rock mass are comparatively small, and the content of hard rock forming minerals is relatively high, the wear of side parts of tip progresses more rapidly rather than that of pointed end of tip. On the other hand, for high index of rock mass strength for wear, the wear of pointed end of tip progresses more rapidly rather than that of side parts of tip.

(4) The ripperbility of rock mass are expressed by index of rock mass strength for wear, and in more details, the relation between porosity or coefficient of crack of rock mass and the products of uniaxial compressive strength, contents of hard rock minerals and so on, are shown in modified Fourmaintraux Diagram.

(5) The wear life of rippertip can be estimated by the function of index of rock mass strength for wear, and the in-situ test method of measuring amount of wear of tip is more valuable than that of measuring wear length of tip for estimating the life of tip due to wear.

These field test results may be expected in future to be useful for the design of initial shape of tip for several kinds of rock mass, the manufacture of tip which controlled the wear resistance of all parts of tip, estimating the wear life of tip for the given ripping operation site, and the rational construction management for mechanical excavation of rock mass.

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