MOISTURE SUSCEPTIBILITY OF ASPHALT MIXTURE AND EFFECTIVENESS OF ANTI-STRIPPING ADDITIVE

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A series of laboratory tests was conducted to evaluate the moisture susceptibility of asphalt mixtures, including moisture damage of asphalt-aggregate bond, and the influence of water immersion on the properties of asphalt mixtures. Furthermore, the effectiveness of various anti-stripping additives was investigated through a variety of tests. It was found that both the asphalt and aggregate types have dominating influences on the moisture susceptibility of asphalt mixtures; hydrated lime slurry is an effective anti-stripping additive for improving the moisture susceptibility; and the net adsorption test, Lottman test and immersed wheel tracking test are effective tools to evaluate the moisture susceptibility.

Key Words: asphalt mixture, moisture susceptibility, laboratory test, asphalt acid value, anti-stripping additive

1. INTRODUCTION

Damage to asphalt mixtures by water is one kind of asphalt pavement failure. The moisture damage occurs in two ways, softening and stripping; the former is characterized by a loss of cohesion, which reduces the strength and stiffness of the asphalt mixture, whereas the latter involves a loss of adhesion and physical separation of asphalt and aggregate¹⁾. In the latter case, which has a greater influence on the pavement performance, adhesion of asphalt deteriorates, asphalt film segregates from the aggregate surface, and finally raveling of the asphalt mixture and potholes generally appear.

This moisture susceptibility of asphalt mixture is related to the aggregate properties, asphalt properties and film thickness, interaction between asphalt and aggregate, air void of asphalt mixture, and traffic²⁾. Among these, the interaction between asphalt and aggregate is the main factor influencing the stripping of asphalt mixtures, so it is very

important to analyze it.

Through chemisorption, which is the process of forming an adsorptive layer in the surface of aggregate through chemical reaction between acidic media in asphalt and cationic media on the surface of aggregate, adhesion between asphalt and aggregate generally becomes larger than physical adsorption, and so the asphalt mixture has much greater moisture resistance³⁾. The effectiveness of chemisorption, that is, the adhesion between asphalt and aggregate, depends on the chemical composition of asphalt and aggregate⁴⁾; for example, asphaltic acid and asphaltic acid anhydride in asphalt make chemical adsorption with high valency charge media salt from the aggregate. The surface active composition in asphalt can be ranked according to activity as follows: asphaltic acid > asphaltic acid anhydride > resin > oil. Thus, the influence of acid materials in asphalt on the adhesion between asphalt and aggregate should be examined further.

The potential for asphalt mixtures to suffer moisture damage can be controlled or reduced through material selection, mixture design that includes high asphalt film thickness and usage of additives, pavement design, construction and drainage^{5), 6)}.

The aggregates and asphalt for mixtures susceptible to stripping can be treated with a variety of anti-stripping additives; these additives commonly include the following:

- Liquid anti-stripping additives

These materials, which are commercially available under various trade names and designations, are used as additives to asphalt. They are easy to handle from a construction standpoint and have been used for a number of years. Their usefulness is, however, somewhat uncertain because of heat stability problems⁷⁾.

- Portland cement

Portland cement is added to aggregates. Though it has been reported to be generally effective, it has not been used widely.

- Hydrated lime

Hydrated lime is normally added to aggregates and has been used widely in many countries. The use of lime as an anti-stripping chemical has long been recognized, and recent researches and field observations have confirmed its usefulness. Many researches indicate that the most efficient method of applying lime is in the form of slurry placed on the surface of aggregates^{8), 9)}.

While hydrated lime is considered as an effective additive to minimize moisture damage, many liquid anti-stripping additives are also recognized to be acceptable and in many cases appear to be equal or better than hydrated lime. It was reported recently that the indirect tensile strength and tensile strength ratio (TSR) for mixtures with liquid anti-stripping additives were not significantly different from those of mixtures containing aggregates fully coated with hydrated lime, mixtures with hydrated lime showed no superiority over those with liquid anti-stripping additives, based on TSR values at several overlay sections containing various anti-stripping additives, and liquid anti-stripping additives can be used as a substitute for hydrated lime without sacrificing moisture resistance¹⁰⁾. Thus, the effect of anti-stripping additives should be examined in detail.

In this study, we examined the fundamental properties of asphalt-aggregate interactions, including adsorption and moisture damage by various test methods, and the influence of asphalt acid value on moisture susceptibility of asphalt

mixture. We also investigated the effectiveness of hydrated lime and other selected anti-stripping additives through various tests, and then evaluated suitable methods to clarify the moisture susceptibility of asphalt mixture.

2. SCOPE OF STUDY

(1) Study plan

In the study on the influence of asphalt acid value on water susceptibility of asphalt mixture, various asphalt mixtures were prepared by using five kinds of asphalt from different crude oil and three kinds of aggregate with different chemical properties.

Two kinds of laboratory test were conducted. One was a series of tests for clarifying the ease of asphalt stripping between asphalt and aggregate, including the water immersion test, electro-optic colorimetry test, and net adsorption test. The other was a series of tests for investigating the change of mechanical properties of asphalt mixture due to water immersion, in which the retained Marshall stability test and Lottman test were conducted.

In the study on the effectiveness of anti-stripping additives for preventing water damage in asphalt mixture, three kinds of anti-stripping additive were used together with a kind of asphalt and three kinds of aggregates.

In this study, the changes of mechanical properties of asphalt mixtures due to water immersion were investigated through laboratory tests. In addition to the aforementioned retained Marshall stability test and Lottman test, the water immersed wheel tracking test was also performed.

Furthermore, the stability of asphalt mixtures treated with anti-stripping additives under the aged condition was evaluated through the Lottman test and the retained Marshall stability test. Two aging methods, that is, short-term aging and long-term aging, were used for asphalt mixtures with limestone aggregates.

(2) Materials

a) Asphalt

Five kinds of asphalt were used in the study on the influence of asphalt on the moisture susceptibility of asphalt mixture. They were commercially produced in different asphalt refineries with different crude oil and were herein denoted as KL, LH, MM, SJ and SL after the refinery. Their properties, including both physical and chemical properties, are shown in **Table 1**. As a chemical property, the asphalt acid value was

Table 1 Properties of asphalts

a) Original

Item	KL	LH	MM	SJ	SL
Density (15°C), g/cm ³	0.9815	1.005	1.013	0.9978	1.001
Solution, %	99.10	99.53	99.91	99.08	99.78
Wax content, %	1.28	3.85	4.08	4.19	5.55
Flash point, °C	300	280	350	320	296
Fraass breaking point, °C	-15	-19.8	-13	-14.8	-16.7
Softening point, °C	49.5	46	50	50.5	49
Penetration (25°C), 1/10 mm	89	138	81	97	96
PI	0.19	-1.43	-0.8	-1.07	-1.69
Ductility (15°C), cm	>150	>150	>100	58	22
Viscosity (60°C), pa·s	309.9	65	129.1	62.4	59.3
Acid value, ml·mol/L/g	2.510	1.256	0.273	1.789	0.564

b) After thin film oven test (TFOT)

Item	KL	LH	MM	SJ	SL
Weight loss, %	-0.05	-0.20	0.15	-0.06	0.10
Softening point, °C	51.5	48.5	52	53	52
Penetration ratio, %	75.2	60.9	66.7	60.8	64.6
Ductility (15°C), cm	>100	43.6	29.8	12.8	6.8

Figure 1 Central value of specified aggregate gradations**Table 2** Physical properties of aggregates

Aggregate	Specific gravity, g/cm ³	Absorption, %	SiO ₂ content, %
Granite	2.751	0.43	76.5
Schist	2.834	0.49	63.1
Limestone	2.712	0.51	6.5
Filler	2.745	—	—

a) AST-3

b) PA-1

Figure 2 Chemical constituents of additives

measured based on reference 11); the asphalt acid value was ranked as KL>SJ>LH>SL>MM.

KL asphalt was widely used in China and was also used in the study on anti-stripping additive to generalize the results.

b) Aggregate

Three kinds of aggregates were used in asphalt mixtures: limestone, granite and schist, the chemical property of which is characterized as basic, acidic, neutral, respectively. Granite has a history of producing water-susceptible asphalt mixture, whereas limestone has good stripping resistance in asphalt mixture.

The central value of the specified gradation and

physical properties of these aggregates are shown in **Figure 1** and **Table 2**, respectively.

c) Additives

Four kinds of anti-stripping additives were evaluated: Portland cement, hydrated lime, and two liquid anti-stripping additives.

The Portland cement and hydrated lime were added in the ratio of 1% of aggregate by mass. Two liquid anti-stripping additives, named AST-3 and PA-1 (commercially available product), were used, the chemical constituents of which are shown in **Figure 2**. The contents were 0.3% of asphalt by mass.

Table 3 Anti-stripping additives		
ID	Additive	Amount
A	Portland cement	1% of aggregates
B	Hydrated lime	1% of aggregates
C	Lime slurry	1% of aggregates
D	AST-3	0.3% of asphalt
E	PA-1	0.3% of asphalt
F	No	—

There were two methods of introducing lime into asphalt mixture:

- i) Dry lime was added to cold aggregates and mixed well to coat the aggregates, and then they were heated before mixing with asphalt.
- ii) Lime, combined with water in the proportion of 7 to 3, was mixed with cold aggregates and they were dried at 130°C for at least 15 hours before mixing with asphalt.

Portland cement was introduced by the dry lime treatment method.

Liquid anti-stripping additives were added to asphalt preheated to about 140°C and they were continuously stirred to ensure homogeneity before mixing with aggregates.

Table 3 summarizes the anti-stripping additives used. The identifications given in the table are used in the following figures.

d) Asphalt mixture

The asphalt mixture was designed in principle by following the standard Marshall method specified for road pavement: 75 blows on both sides of the specimen in preparation and a Marshall stability of 7.5 kN. For asphalt mixtures used to study moisture susceptibility, the asphalt content of 5%, which was not the optimum asphalt content for all the mixtures, was fixed to eliminate the effect of asphalt content on the properties of asphalt mixtures. However, the optimum asphalt content of 4.8% was adopted for those used to study the effect of anti-stripping additives, but was not adjusted for the presence or absence of additives.

(3) Test methods

a) Stripping resistance of asphalt-aggregate bond

The following three kinds of tests were performed.

- Water immersion test

This test visually determines the extent of stripping of asphalt from aggregate surfaces after the asphalt mixture is immersed in water for a specified time in accordance with JHS 201¹²⁾. The extent of stripping, namely stripping ratio, is quantified as the percentage of asphalt lost in the

mixture during water immersion.

- Electro-optic colorimetry test

This test quantifies the stripping susceptibility of asphalt-aggregate by measuring the difference in phenol safranine dyestuff solution after both aggregate and asphalt-aggregate are soaked. In the test, clean aggregates of 2.5 - 5 mm in diameter mixed with asphalt and pure aggregates are immersed separately into phenol safranine dyestuff solution at 60°C for two hours. Then, the concentration is measured using spectrophotometer at the wavelength of 510nm. The test method substantially follows the general solution analytical technique described in reference 3), for instance.

The stripping ratio is calculated as q'/q (q : pure aggregate adsorption (mg/g) and q' : asphalt-aggregate adsorption).

- Net adsorption test

The test determines the affinity between asphalt and aggregate (initial adsorption) and the moisture sensitivity of the asphalt-aggregate bond (net adsorption), by using asphalt-toluene solution, as specified by SHRP M-001¹³⁾. The initial adsorption is determined from the difference in concentration of asphalt-toluene solution before and after contact with aggregates. The net adsorption is also determined from the difference in concentration of asphalt-toluene solution before and after contact with aggregates together with water immersion. The stripping ratio is calculated in the same way as mentioned above.

In the electro-optic colorimetry test, the phenol safranine dyestuff can soak into the interface between aggregate and asphalt, and the asphalt film is not peeled away but separated from the aggregate surface. In contrast, the asphalt film might be peeled away from the aggregate surface in other tests.

b) Influence of water on asphalt mixture properties

Three types of tests were used to evaluate the influence of water immersion on the mechanical properties of asphalt mixtures, as follows.

- Retained Marshall stability test

The retained Marshall stability (S_0) is calculated as the ratio of Marshall stability of specimen soaked in water for forty-eight hours at 60°C (S_1) to that soaked in water for two hours at 60°C (S_2), specified by JHS 202.

- Lottman test

Indirect tensile strength is used. Moisture susceptibility is determined by the ratio of indirect tensile strength in the wet condition (R_2) to that in the dry condition (R_1) as the tensile strength ratio (TSR), in accordance with AASHTO T283.

Table 4 Stripping ratio in water immersion test

Aggregate	KL	LH	MM	SJ	SL
Granite	50	65	55	60	70
Limestone	5	5	15	5	15
Schist	15	20	35	20	70

(unit: %)

Table 5 Stripping ratio in electro-optic colorimetry test

Aggregate	KL	LH	MM	SL	SJ
Granite	35.6	85.3	57.4	96.7	54.9
Limestone	22.2	72.6	39.2	64.7	43.1
Schist	48.7	64.3	42.9	84.4	34.6

(unit: %)

Table 6 Stripping ratio in net adsorption test

Asphalt	Granite			Limestone			Schist		
	Initial adsorption	Net adsorption	Stripping ratio	Initial adsorption	Net adsorption	Stripping ratio	Initial adsorption	Net adsorption	Stripping ratio
KL	0.0618	0.0153	75.2	0.0834	0.0770	8.0	0.2252	0.1182	19.7
LH	0.0318	0.0127	60.1	0.8055	0.0697	15.5	0.1949	0.1223	37.9
MM	0.0469	0.0094	79.9	0.1080	0.0685	30.3	0.1391	0.0529	62.0
SJ	0.1196	0.0490	58.9	0.1057	0.0799	17.0	0.1732	0.1366	21.2
SL	0.0097	0.0	100.0	0.1565	0.1069	31.1	0.1432	0.0606	57.7

(unit: original adsorption-mg/g, net adsorption-mg/g, stripping ratio-%)

Table 7 Criteria on moisture susceptibility¹⁴⁾

Stripping ratio (%)	Asphalt-aggregate adhesion
<30	Good
45-30	Marginal
>45	Poor

- Immersed wheel tracking test

The wheel tracking test is performed on the specimen immersed in water at 60°C for forty-eight hours. The stripping index (SI) is calculated as the ratio of the loss of mass during the test to the mass before the test.

Two aging methods, which are partly applied before preparing the specimens for the above-mentioned tests, are as follows:

- Short-term aging method

Loose asphalt mixture with an even thickness of 25–50 mm is placed in a forced-draught oven at 135°C for four hours, stirred every hour, and then the specimens are prepared for the tests.

- Long-term aging method

Specimens are prepared by using asphalt mixtures aged briefly in the above-mentioned way and placed in an oven at 85°C for 120 hours before the tests.

3. WATER SUSCEPTIBILITY OF ASPHALT MIXTURES

This section reveals the properties of asphalt-aggregate interactions and the influence of asphalt acid value on moisture susceptibility of asphalt mixtures through various tests.

(1) Stripping resistance of asphalt-aggregate bond

The results of the water immersion test are shown in **Table 4**. The stripping ratio depends on both the type of aggregates and the type of asphalt. Whereas limestone showed good resistance against stripping, granite was very susceptible to stripping. From the viewpoint of stripping resistance, asphalt is ranked as KL>SJ>LH>MM>SL.

The results of the electro-optic colorimetry test are shown in **Table 5**. The influence of both the type of aggregate and the type of asphalt on the moisture susceptibility of asphalt mixtures is similar to that in the water immersion test.

The results of the net adsorption test are shown in **Table 6**, indicating a similar trend to moisture susceptibility. The chemical properties of aggregates play a significant role in determining the strength of bond; the stripping ratio ranges from 8.0 to 31.3% with limestone, from 19.7 to 62.0% with schist and from 58.9 to 100% with granite.

According to the criteria on moisture susceptibility of asphalt-aggregate adhesion suggested by SHRP as shown in **Table 7**, limestone is evaluated to have superior stripping resistance, and schist is also evaluated to have marginally higher stripping resistance than granite.

The types of asphalt also have a large influence on moisture susceptibility. Whereas KL, LH and SJ asphalts appear to have higher stripping resistance, MM and SL asphalts appear to have lower stripping resistance.

The stripping ratios measured in these three tests are plotted against asphalt acid value, which represents the chemical properties of asphalt, in **Figures 3, 4 and 5**. These figures show that the

Table 8 Retained Marshall stability

Asphalt	Granite			Limestone			Schist		
	S_1	S_2	S_0	S_1	S_2	S_0	S_1	S_2	S_0
KL	10.63	6.42	63.5	8.27	7.87	90.3	10.61	7.88	74.3
LH	10.12	4.40	43.4	7.99	7.41	89.4	11.74	6.42	54.7
MM	10.70	0	0	10.63	8.01	79.9	11.95	2.94	24.6
SJ	10.11	6.46	63.9	8.96	7.28	81.3	10.96	8.03	73.3
SL	10.39	2.16	20.8	8.83	7.82	88.5	11.98	5.28	44.1

(unit: S_1, S_2 -kN, S_0 -%)**Table 9** Tensile strength ratio in Lottman test

Asphalt	Granite			Limestone			Schist		
	R_1	R_2	TSR	R_1	R_2	TSR	R_1	R_2	TSR
KL	0.564	0.217	38.5	0.465	0.450	96.8	0.482	0.353	73.2
LH	0.515	0.158	30.7	0.516	0.394	75.4	0.527	0.250	47.4
MM	0.621	0.077	12.1	0.671	0.385	57.4	0.637	0.092	14.8
SJ	0.624	0.210	33.7	0.483	0.429	88.8	0.638	0.364	57.1
SL	0.576	0.037	6.4	0.459	0.351	76.5	0.616	0.173	28.1

(unit: R_1, R_2 -MPa, TSR-%) **Figure 3** Acid value and stripping ratio in water immersion test **Figure 5** Acid value and stripping ratio in net adsorption test **Figure 4** Acid value and stripping ratio in electro-optic colorimetry test

stripping ratios decrease as the asphalt acid value increases. As the water immersion test involved visual observation, the results may contain significant errors. The electro-optic colorimetry test, which has disadvantages such that the stripping ratio

does not clearly differ among the asphalt mixtures, might show the separation ratio of asphalt film from the aggregate surface.

In contrast, the net adsorption test evaluates asphalt adsorption from the aggregate surface by directly measuring changes in the concentration of asphalt solution, and so can reduce the error. Thus, this method is suitable for evaluating the moisture susceptibility of asphalt-aggregate bond.

(2) Mechanical properties of asphalt mixtures

The results of the retained Marshall stability test are shown in **Table 8**. Of the aggregates, limestone has superior moisture stability, whereas granite has the lowest moisture stability. The moisture stability of the asphalt mixture with limestone, which is influenced by asphalt type, is ranked as KL>LH>SL>SJ>MM, and the stability with other aggregates is similar to this.

The results of the Lottman test are shown in

Figure 6 Asphalt acid value and retained Marshall stability

Figure 7 Asphalt acid value and TSR

Figure 8 Anti-stripping additives and TSR

Figure 9 Anti-stripping additive and retained Marshall stability

Table 9. The influence of aggregate type on the moisture susceptibility is the same as that in the retained Marshall stability test. According to the recommendation¹⁵⁾, the asphalt mixture with limestone, having TSR of 70% or more, meets the specification except for MM asphalt, but the mixtures with other aggregates do not meet the specification. The asphalt mixture with KL asphalt has the highest moisture stability, whereas those with MM and SL asphalt have the lowest.

The relationship between asphalt acid value and moisture susceptibility of asphalt mixture is shown in Figures 6 and 7.

Figure 6 shows that the retained Marshall stability increases with asphalt acid value and depends on the type of aggregate. These trends are similar to those for asphalt-aggregate bond. As the retained Marshall stability of asphalt mixture with limestone scarcely changes with asphalt acid value, the retained Marshall stability test could hardly distinguish the moisture susceptibility in all the aggregate cases.

In Figure 7, TSR of the asphalt mixture with limestone increases with asphalt acid value. Thus, the Lottman test is suitable for evaluating the moisture susceptibility of asphalt mixtures.

4. EFFECT OF ADDITIVES ON MOISTURE SUSCEPTIBILITY

This section clarifies the effect of anti-stripping additives on the moisture susceptibility by a series of laboratory tests, and ensures the applicability of additives for actual construction through two aging methods.

(1) Property changes due to water immersion

As shown in Figure 8, the results of the Lottman test show that all the anti-stripping additives improve the moisture susceptibility of asphalt mixtures and the degree of improvement depends on both the additive and aggregate types.

From the figure, it is found that the lime slurry (C) has the highest resistance to moisture damage and hydrated lime (B) and liquid anti-stripping additive AST-3 (D) also have good moisture stability. However, Portland cement (A) slightly improves the moisture susceptibility of asphalt mixture. In addition, the aggregate strongly influences the moisture stability after adding the additives; that is, TSR of the asphalt mixture with granite never exceeds that of mixtures with other aggregates.

Figure 10 Rut depth with tracking cycles (Granite)

Figure 11 Rut depth with tracking cycles (Schist)

Figure 12 Anti-stripping additive and rut depth

Figure 13 Anti-stripping additive and SI

Figure 14 Influence of aging on retained Marshall stability

Figure 15 Influence of aging on TSR

Figure 9 shows the retained Marshall stability. From the figure, it is found that all the anti-stripping additives except Portland cement (A) have basically similar effects on improving the moisture susceptibility of asphalt mixtures. Thus, the retained Marshall stability test yields almost no difference even when the treatment method changes.

Figures 10 and 11 show the results of the immersed wheel tracking tests for granite and schist aggregates, respectively. From these figures, it is found that the rut depth increases with tracking cycles irrespective of aggregate type, and that the asphalt mixture with schist has better rutting and water resistance than that with granite.

Figure 12 shows the rut depth at the end of the test. The asphalt mixtures with schist show smaller rut depth than those with granite, despite the larger number of tracking cycles. Lime, regardless of whether dry or slurry (B and C), remarkably improves the moisture stability of asphalt mixture. On the contrary, Portland cement (A) and liquid anti-stripping additives (D and E) have less effect.

The stripping index SI indicates that lime slurry (C) can significantly improve the moisture susceptibility of asphalt mixture, as shown in Figure 13. Portland cement (A) has little effect as with rut depth. Thus, the results of the immersed

wheel tracking test show the same influence of aggregate type on moisture susceptibility as the results of the Lottman test.

(2) Influence of aging method on moisture susceptibility

Figure 14 shows the retained Marshall stability test results of limestone asphalt mixture. The retained Marshall stability for each anti-stripping additive decreases with the aging process. As to the difference in the retained Marshall stability among the anti-stripping additives, it is clear without aging, while it becomes imperceptible after long-term aging.

Figure 15 shows TSR in the Lottman test for limestone asphalt mixture. TSR values increase after short-term aging irrespective of treatment type, due to stiffening of the asphalt concrete with aging, but decrease after long-term aging. It is also found that lime (B) is more effective than the liquid anti-stripping additive (D) even after long-term aging. Thus, the Lottman test would be more useful than the retained Marshall stability test in evaluating the moisture susceptibility.

5. SUMMARY AND CONCLUSION

A series of laboratory tests was conducted to evaluate the moisture susceptibility of asphalt mixtures and the effectiveness of anti-stripping additives. The following conclusions were drawn.

1. Both the asphalt and aggregate types have dominating influences on the asphalt-aggregate bond and moisture susceptibility of asphalt mixtures.
2. Asphalt acid value can be used to evaluate the moisture susceptibility of asphalt mixtures; the moisture stability increases with asphalt acid value.
3. Limestone has superior stripping resistance and schist has marginally higher stripping resistance than granite.
4. The net adsorption test is superior to the water immersion and electro-optic colorimetry test for evaluating asphalt-aggregate bond.
5. The treatment of asphalt mixtures with hydrated lime slurry is a more effective method for decreasing moisture susceptibility, in comparison with liquid anti-stripping additives and Portland cement.
6. The Lottman test and immersed wheel tracking

test are better than retained Marshall stability test for evaluating the moisture susceptibility of asphalt mixtures.

7. The moisture susceptibility of asphalt mixture with additives decreases with the aging process. The difference in the moisture susceptibility among the aggregates after long-term aging process could be evaluated by the Lottman test.

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